

FAA Conference 2015
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Oldcastle Materials

SUPERPAVE TRAINING

SUPERPAVE

- SUPERPAVE the Industry standard
 - Initial Projects were let statewide in 1996
 - 100% SUPERPAVE Implementation on projects ***let*** in 1999



SUPERPAVE Issues

- Specification encourages coarse mixes with low asphalt contents
 - Difficulty w/Compaction
 - Prone to Segregation



Superpave FAA

- P401- Marshall Specification
 - Served the FAA well
 - EB 59
 - Struggled with Nequivalent



SUPERPAVE

- "SYSTEM"
 - Mix Design
 - Aggregate Specs
 - PGB Binder Criteria
 - ESAL's
 - Gyratory Compactor
 - Volumetric Analysis
 - Compaction Requirements



Features of P401 Superpave

- Superpave

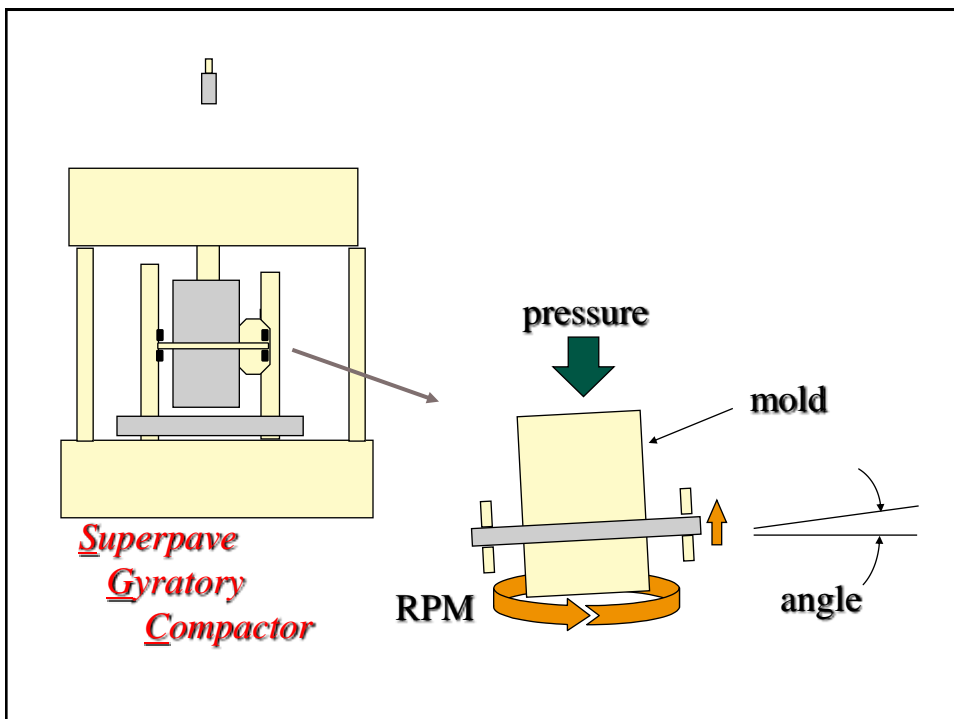


Gyratory Compactor

- Gyration is a function of Mix ESAL's
 - MORE ESAL's- More gyrations
 - LESS ASPHALT (Rutting)
- Simulate the effect of traffic after over the "life cycle"



California State University



Gyratory Compactor

- Sample size
 - 115 mm +/- 5 mm
 - Typically 4600 grams
- Number of Specimens
 - 3-5 Specimens
- Ndesign
 - 75 > 60,000 lbs
 - 50 < 60,000 lbs



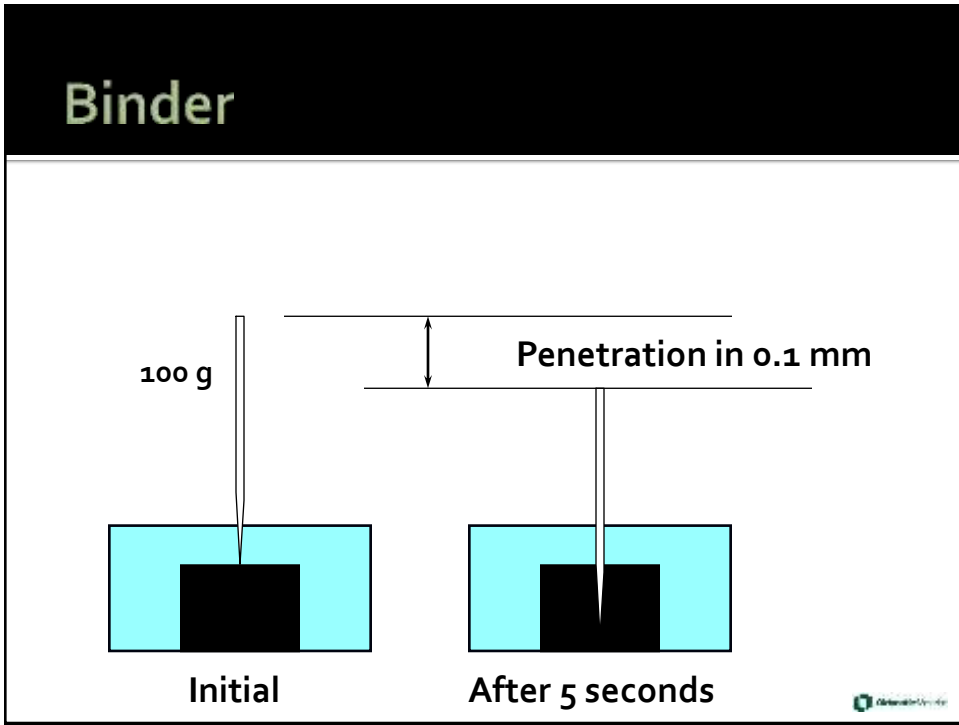
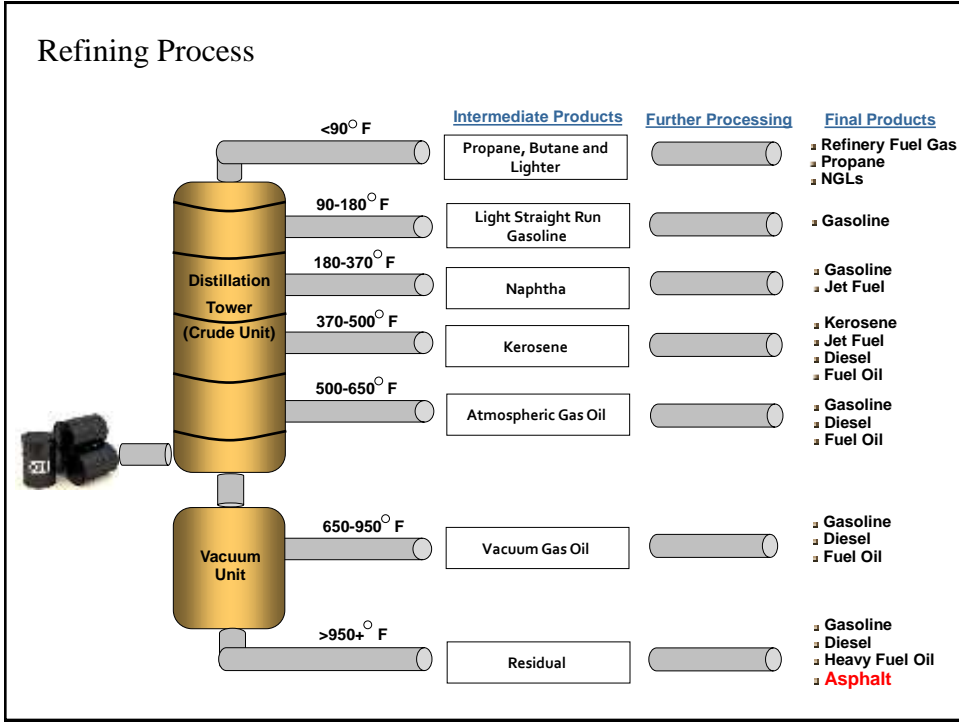
 Mettler-Toledo

Stability and Flow

- Requirements are removed for Superpave option

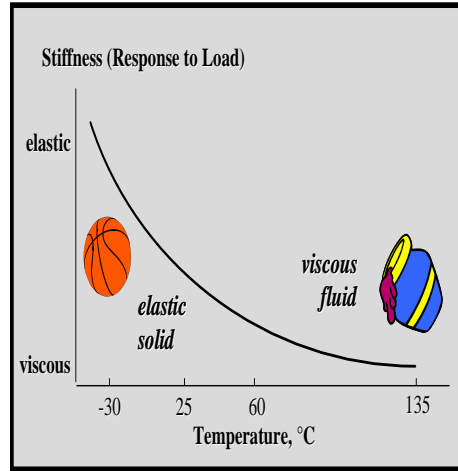


 Mettler-Toledo



Binder

- Specifications evolved to characterize asphalt binder's viscosity in the 1960's.
- Viscosity is defined as resistance to flow
 - Asphalt is "temperature viscous"
 - Temperature of the material affects its stiffness (or resistance to flow)



University of Michigan

Binder Tests

- Need to further characterize asphalt binder due to widespread pavement failures (permanent deformation)
 - Radial tires
 - Increased truck traffic
 - Higher tire pressures
 - Changes in refining process



University of Michigan

Superpave Binder

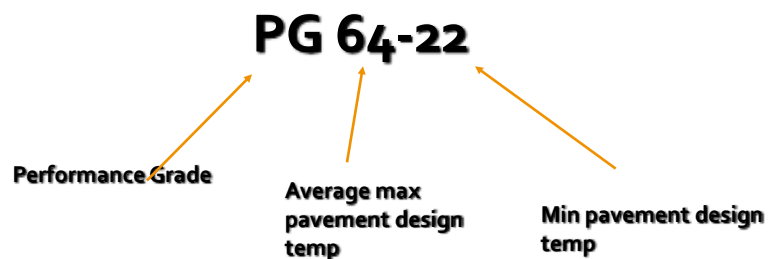
- Rolling Thin Film Oven Test
- Pressure Aging Vessel
- Direct Tension Tester
- Dynamic Shear Rheometer
- Bending Beam Rheometer



Direct Tension Bath and Load

Binder

Superpave Binder specification grading based on pavement climatic conditions from LTPP bind



Binder

- PG System
 - Grade bumping
 - < 12,500 Standard
 - <100,000 – 1 grade
 - > 100,000- 2 grade
 - “Plus” specifications
 - Elastic recovery to ensure that material polymer modified (70%)
- Highly polymerized materials that are difficult to use at reasonable temperatures



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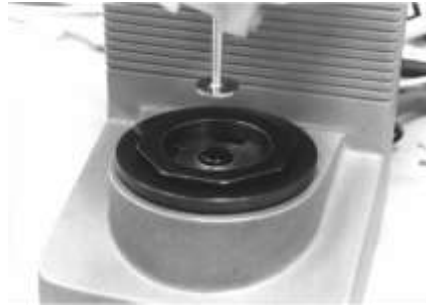
MSCR Binder

- MSCR
 - Multiple Stress Creep Recovery
 - AASHTO TP 70
- DSR
 - High and intermediate temperature grading
- NEW Grading for asphalt
- PG Grading appropriate for grading, followed by “letter” designation
- Changes to AASHTO M320, Table 1
 - PG 64-22S
 - Standard traffic loading
 - PG 64-22H
 - Heavy traffic loading
 - PG 64-22V
 - Very heavy traffic loading
 - PG 64-22E
 - Extremely heavy traffic loading

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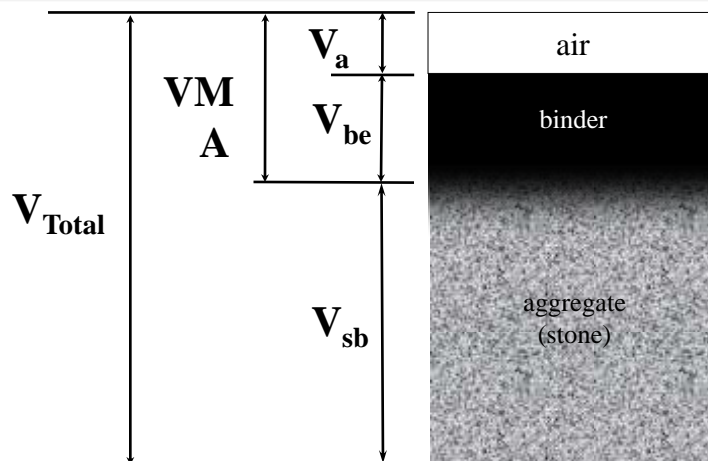
Binder

- MSCR Implementation
 - Transition from AASHTO M 320 to AASHTO MP 19
- Regional User-Producer groups and agencies
- Expect some changes in binder



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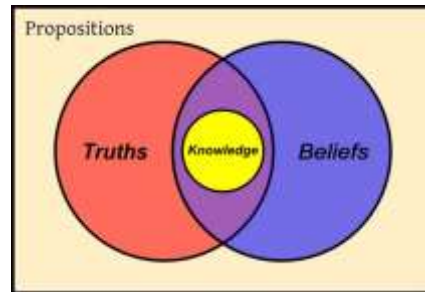
Voids in Mineral Aggregate (VMA)



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VMA Historical perspective

- 1901- Referenced in FJ Warren patent application
- Early Marshall looked at VFA only (65-85%)
- 1956- McLeod proposes minimum VMA of 15% (based on 5% air voids and also revised in 1959 to differentiate NMAAS)
- 1962- Asphalt Institute proposes McLeod recommendations and also drops VFA in MS-2 (AI reinstates VFA in 1992)
- Post 1962- Line in the sand...
- VMA vs. Film Thickness



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VMA

$$\text{VMA} = 100 - \frac{G_{mb} P_s}{G_{sb}}$$

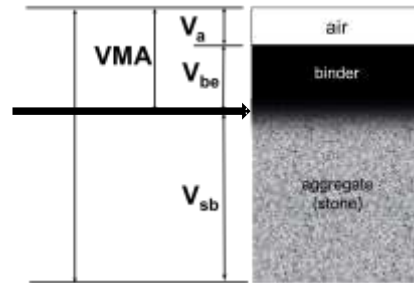
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VMA is the volume of air voids and effective binder (binder not absorbed into the aggregate), and in design is a powerful parameter to indicate that sufficient volume of binder is in the mixture

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Volumetrics- VMA

- VMA
 - Excludes the portion of binder absorbed into the aggregate
 - As nominal maximum size decreases VMA increases (more volume of binder required to coat higher amount of surface area in finer mixtures)



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Voids Filled with Asphalt

$$\text{VFA} = 100 \times \frac{\text{VMA} - V_a}{\text{VMA}}$$

VFA is the percent of VMA that is filled with asphalt cement (related to “saturation” from soil mechanics)

M 323 Requirements for VMA

Table 6—Superpave HMA Design Requirements

Design ESALs ^a (Million)	Required Relative Density, Percent of Theoretical Maximum Specific Gravity			Voids in the Mineral Aggregate (VMA), Percent Minimum						Voids Filled with Asphalt (VFA) Range, ^b Percent	Dust-to- Binder Ratio Range ^c
	$N_{initial}$	N_{design}	N_{max}	Nominal Maximum Aggregate Size, mm							
				37.5	25.0	19.0	12.5	9.5	4.75		
<0.3	≤91.5	96.0	≤98.0	11.0	12.0	13.0	14.0	15.0	16.0	70–80 ^d	0.6–1.2
0.3 to <3	≤90.5	96.0	≤98.0	11.0	12.0	13.0	14.0	15.0	16.0	65–78	0.6–1.2
3 to <10	≤89.0	96.0	≤98.0	11.0	12.0	13.0	14.0	15.0	16.0	65–75 ^e	0.6–1.2
10 to <30	≤89.0	96.0	≤98.0	11.0	12.0	13.0	14.0	15.0	16.0	65–75 ^e	0.6–1.2
≥30	≤89.0	96.0	≤98.0	11.0	12.0	13.0	14.0	15.0	16.0	65–75 ^e	0.6–1.2

^a Design ESALs are the anticipated project traffic level expected on the design lane over a 20-year period. Regardless of the actual design life of the roadway, determine the design ESALs for 20 years.

^b For 37.5-mm nominal maximum size mixtures, the specified lower limit of the VFA range shall be 64 percent for all design traffic levels.

^c For 4.75-mm nominal maximum size mixtures, the dust-to-binder ratio shall be 0.9 to 2.0.

^d For 25.0-mm nominal maximum size mixtures, the specified lower limit of the VFA range shall be 67 percent for design traffic levels <0.3 million ESALs.

^e For design traffic levels >3 million ESALs, 9.5-mm nominal maximum size mixtures, the specified VFA range shall be 73 to 76 percent and for 4.75-mm nominal maximum size mixtures shall be 75 to 78 percent.

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VMA

- Recommend that design VMA +0.5% desired production VMA
- Each 1% VMA = 0.4 liquid
- Aggregate characteristics including gradation, surface, texture and shape impact VMA

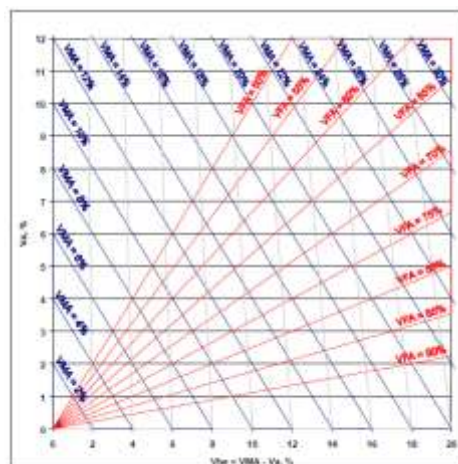


FIGURE 2. Universal Volumetric Chart

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Thanks!!